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# GEOTECHNICAL LOG OF NON-CORE DRILLHOLE

**Borehole No:** BH107

**Sheet No:** 1 OF 2

**Project No:** 3003659

**Client:** QDTMR

**Project:** Smith Olsen Detailed Design Geotech Investigation **Co-ordinates System:** UTM Zone 56

**Feature:** **Eastings:** 535181.8m E

**Location:** Refer Location Plan **Northings:** 6906913.4m S

**Surface RL (m):** 39.67

**Angle from Horz:** 90

**Direction:** n/a

DRILLING							TESTING				SUBSTANCE					
Method	Support	Rate			Water	Sample	Depth (m)	Depth/RL	Type	Sample or Field Test	Graphic Log	USC Symbol	Description	Moisture	Consistency/Density	Other Observations
1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17
WR ATC							39.67 39.57				X	Cl	Clayey SILT: Firm to stiff, low plasticity, dark grey-brown, some organic material i.e. grass roots, moist, topsoil. Gravelly CLAY: Stiff to very stiff, low to medium plasticity, some gravel and cobbles, moist, fill.	M	F St VS	Hole was needed to be moved slightly as large cobbles or boulders were struck at approx 0.2m.
							0.50						Refer to Geotechnical Log of Cored Drillhole			
							1									
							2									
							3									
							4									
							5									
							6									
							7									
							8									
							9									

**Notes (Instrumentation etc):**

<b>Contractor:</b> GeoDrill	<b>Commenced:</b> 28/07/11	<b>Logged By:</b> ME/BD
<b>Equipment:</b> Hydropower Scout	<b>Completed:</b> 28/07/11	<b>Checked By:</b> AR

Basis of description and details of abbreviations are given on explanatory notes

SMEC GOLD COAST BOREHOLE NON CORE LOG I:\PROJECTS\3003659\005\_OPERATIONS\DD15\_GEOTECHNICAL\INVESTIGATIONS\GINT FILES\SMITH - OLSEN BOREHOLES (CURRENT)\GP\_231111



# GEOTECHNICAL INVESTIGATIONS LOG

Borehole No: BH107

Sheet No: 2 OF 2

Project No: 3003659

Client: QDTMR

Project: Smith Olsen Detailed Design Geotech Investigation Co-ordinates System: UTM Zone 56

Feature: E: 535181.8

Location: Refer Location Plan N: 6906913.4

Surface RL (m): 39.67

Angle from Horz: 90

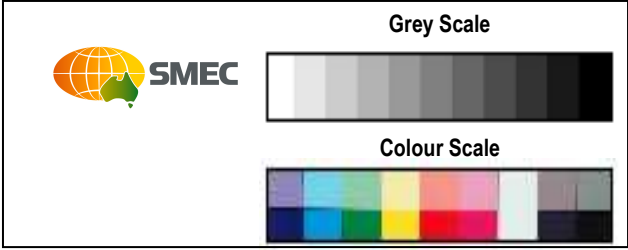
Direction: n/a

DRILLING					SUBSTANCE							TEST		DEFECTS												
Method	Water	TCR %	RQD %	Lift	Depth (m)	Depth/RL	Graphic Log	Description	Weathering				Estimated Strength			Type	Result	Fracture Spacing (mm)	Description							
1	2	3	4	5	6	7	8	9	EW	HW	MW	SW	FS	FR	EL	VL	L	M	H	VH	EH	12	13	14	15	16
																	40	300								
																	20	100	1000							
						Refer to Geotechnical log of Non-cored Drillhole																				
						Start Coring at 0.50m																				
						0.50	GREYWACKE: Very fine grained, blue-grey, very high strength, fresh boulder, fill																			
						0.85	Sandy CLAY: Stiff, medium to low plasticity, fine grained sand, dark grey-brown tending carbonaceous, some high strength gravel and cobbles, moist, fill.																			
						0.92	Stained/bleached red-brown.																			
						1.10	Yellow-brown, some organic rootlets.																			
						1.30	Sandy CLAY: Firm, medium to high plasticity, dark yellow-brown, moist to wet, fill.																			
						1.47	CORE LOSS (1.80m to 1.90m)																			
						1.50	Firm to soft, high plasticity, brown to brown grey, a high strength weathered Siltstone/Sandstone cobble, wet.																			
						1.80	CORE LOSS (2.05m to 2.10m)																			
						1.90	METASANDSTONE: Fine grained, pale grey-brown some iron staining in defects, some thin dark grey siltstone laminations, fractured, medium to high strength, moderately weathered.																			
						2.05	J, 70, In, Cy, Pt, Sm																			
						2.10	CORE LOSS (2.05m to 2.10m)																			
						2.20	J, 55, Ct, Cl, Pt, Sm																			
						2.30	Medium to coarse grained, dark yellow-brown with iron staining, weaker zone, bedding at 55°, very low to low strength, extremely weathered to highly weathered.																			
						2.40	Be, 37, Ct, Cl, Un, Ro																			
						2.50	Fine grained, pale grey-brown, thin siltstone laminations, slightly fractured, medium to high strength, moderately weathered, some grass in defects.																			
						2.60	J, 35, Vn, Fe, Pt, Sm																			
						2.70	J, 22, St, Fe, Un, Sm																			
						2.80	J, 55, Vn, Cy, Dis, Sm																			
						2.90	J, 55, Vn, Ct, Pt, Ro																			
						3.00	J, 55, St, C, Un, Ro																			
						3.10	J, 60, St, C, Un, Ro																			
						3.20	J, 50, In, Cy, Pt, Sm																			
						3.30	J, 45, In, Cy, Pt, Sm																			
						3.40	J, 45, Vn, Ct, Pt, Ro																			
						3.50	J, 80, Ct, Pt, Sm																			
						3.60	J, 20, Vn, Ct, Pt, Ro																			
						3.70	J, 68, Ct, Cl, Pt, Ro																			
						3.80	More frequent siltstone laminations.																			
						3.90																				
						4.00																				
						4.10																				
						4.20																				
						4.30																				
						4.40																				
						4.50																				
						4.60																				
						4.70																				
						4.80																				
						4.90																				
						5.00	Hole discontinued at 5.10m																			
						5.10																				
						5.20																				
						5.30																				
						5.40																				
						5.50																				
						5.60																				
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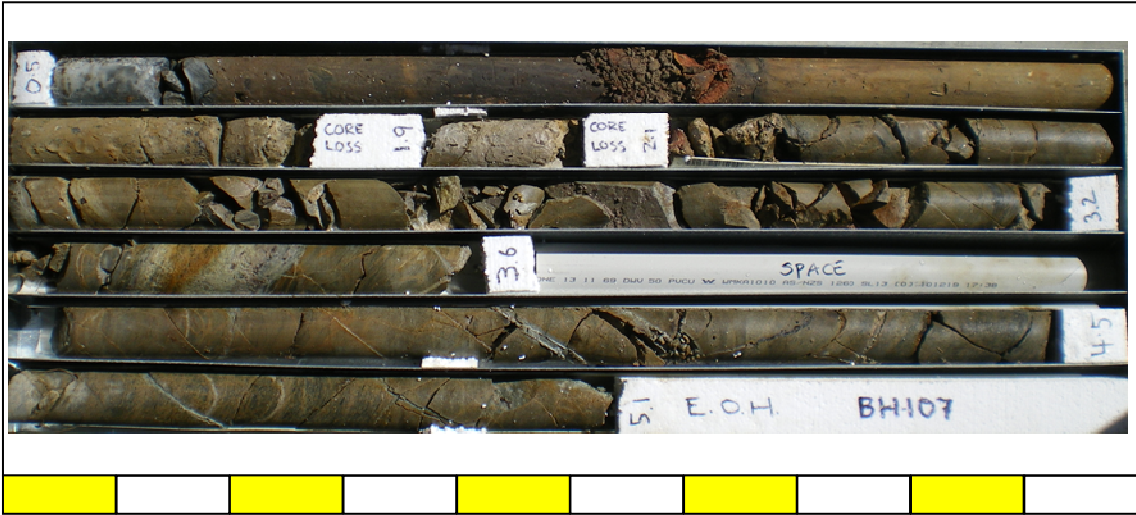
Notes (Instrumentation etc):

Contractor: GeoDrill	Commenced: 28/07/11	Logged By: ME/BD
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Borehole Number		<b>BH107</b>	
Box	1	of	1
Depth	0.5m	to	5.1m
Project	Smith St & Olsen Av		
Number	3003659		
Client	QDTMR		



## NOTES RELATING TO GEOTECHNICAL REPORTS AND SITE INVESTIGATION LOGS

### GEOTECHNICAL REPORTS AND SITE INVESTIGATION LOGS

Geotechnical reports/logs are prepared by qualified personnel on the information supplied or obtained and are based on current engineering standards of interpretation and analysis.

Information may be gained from limited subsurface testing, surface observations, previous work, and is supplemented by knowledge of the local geology and experience of the range of properties that may exhibited by the materials present. For this reason, geotechnical reports should be regarded as interpretative rather than factual documents, limited to some extent by the scope of information on which they rely.

Where the report/log has been prepared for a specific purpose (e.g. design of a three-storey building), the information and interpretation may not be appropriate if the design is changed (e.g. a twenty-storey building). In such cases, the report/log and the sufficiency of the existing work should be reviewed by SMEC in the light of the new proposal.

Every care is taken with the report/log content; however, it is not always possible to anticipate or assume responsibility for the following conditions:

- Unexpected variations in ground conditions. The potential for this depends on the amount of investigative work undertaken.
- Changes in policy or interpretation by statutory authorities
- The actions of contractors responding to commercial pressures

If these occur, SMEC would be pleased to resolve the matter through further investigation, analysis or advice.

### UNFORESEEN CONDITIONS

Should conditions encountered on site differ markedly from those anticipated from the information contained in the report/log, SMEC should be notified immediately. Early identification of site anomalies generally results in any problems being more readily resolved and allows re-interpretation and assessment of the implications for future work.

### SUBSURFACE INFORMATION

Logs of a borehole, recovered core, test pit, excavated face, or cone penetration test are an engineering and/or geological interpretation of the subsurface conditions. The reliability of the logged information depends on the drilling/testing method, sampling/observation spacing's and the ground conditions. It is not always possible or economic to obtain continuous high-quality data. It should also be recognised that the volume of material observed or tested is only a fraction of the total subsurface profile.

Interpretation of subsurface information and application to design and construction must take into consideration the spacing of the test locations, the frequency of observations and testing, and the possibility that geological boundaries may vary between observation points.

Groundwater observations and measurements outside of specially designed and constructed piezometers should be treated with care for the following reasons:

- In low permeability soils groundwater may not seep into an excavation or bore in the short time it is left open.
- A localised perched water table may not represent the true water table.
- Groundwater levels vary according to rainfall events or season.
- Some drilling and testing procedures mask or prevent groundwater inflow.

The installation of piezometers and long-term monitoring of groundwater levels may be required to adequately identify groundwater conditions.